

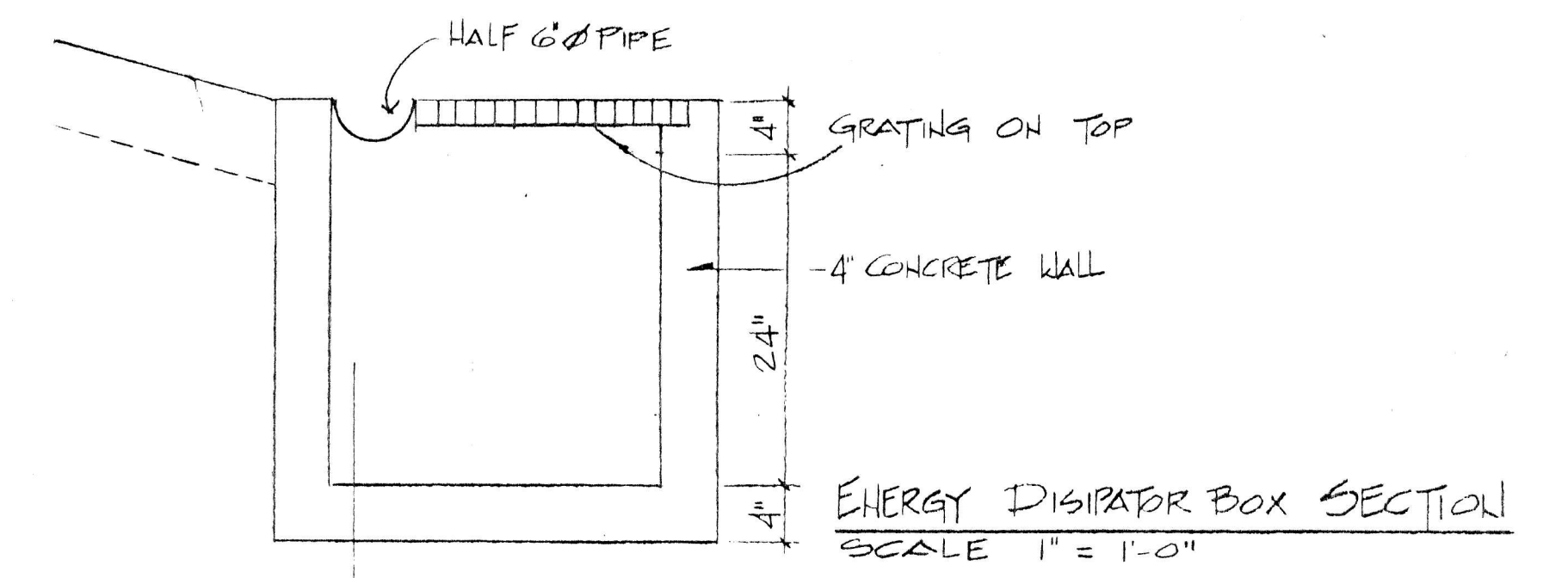
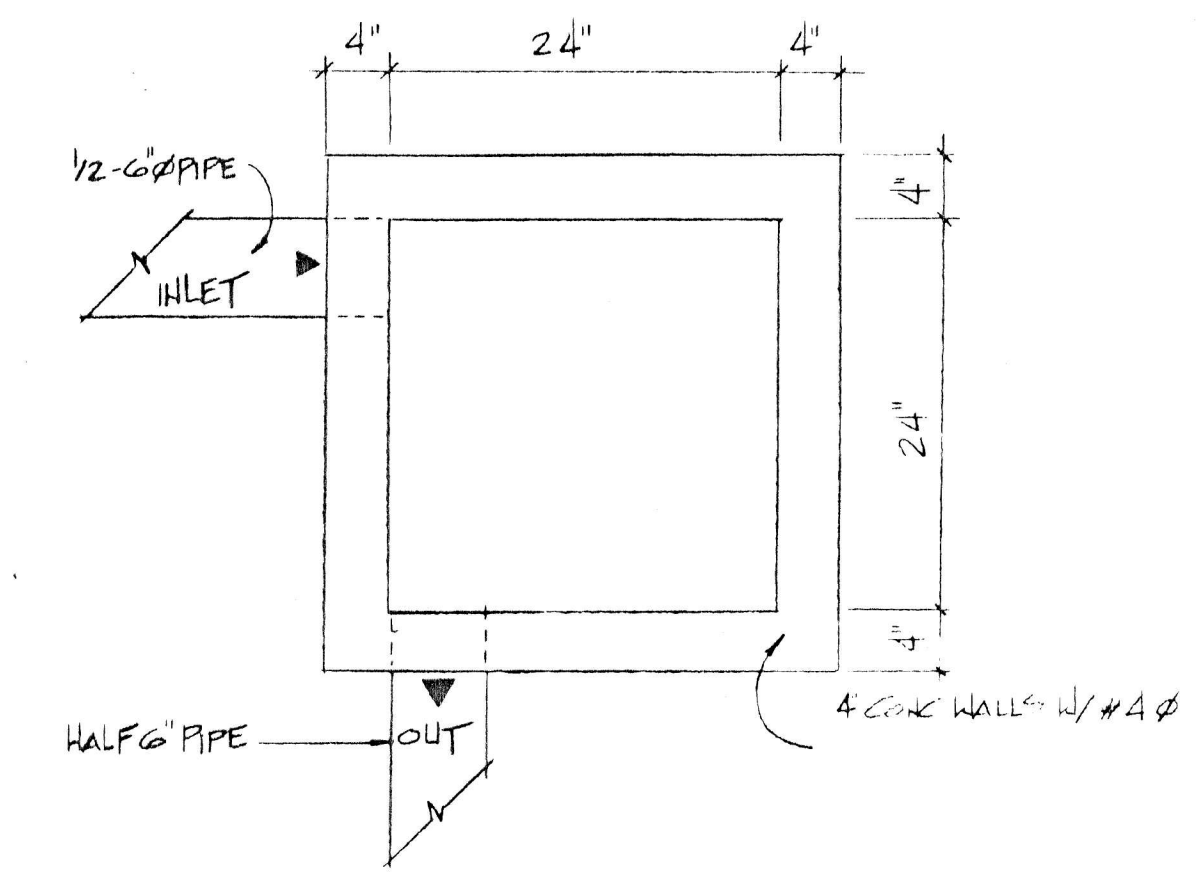
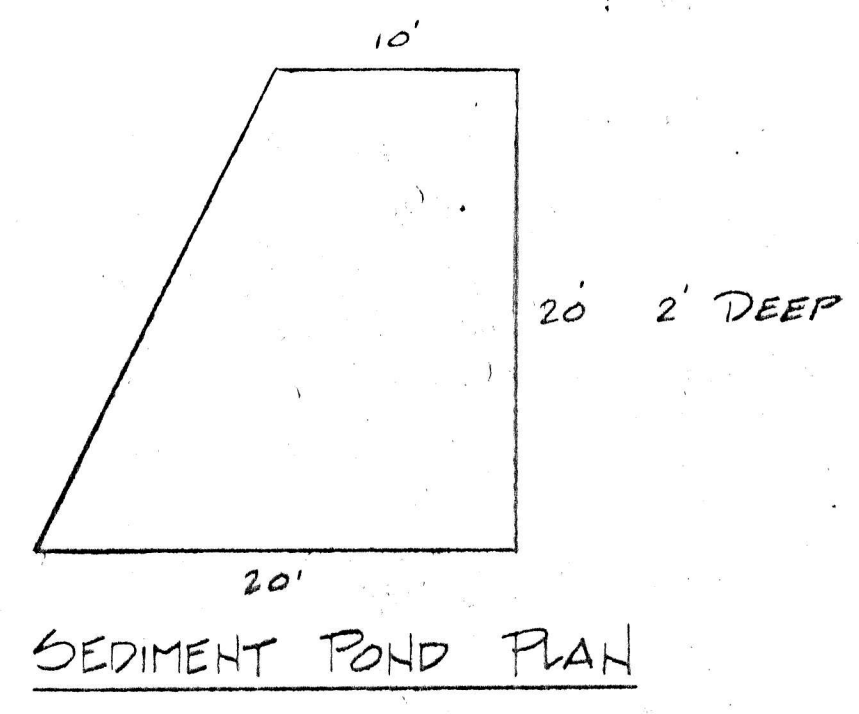
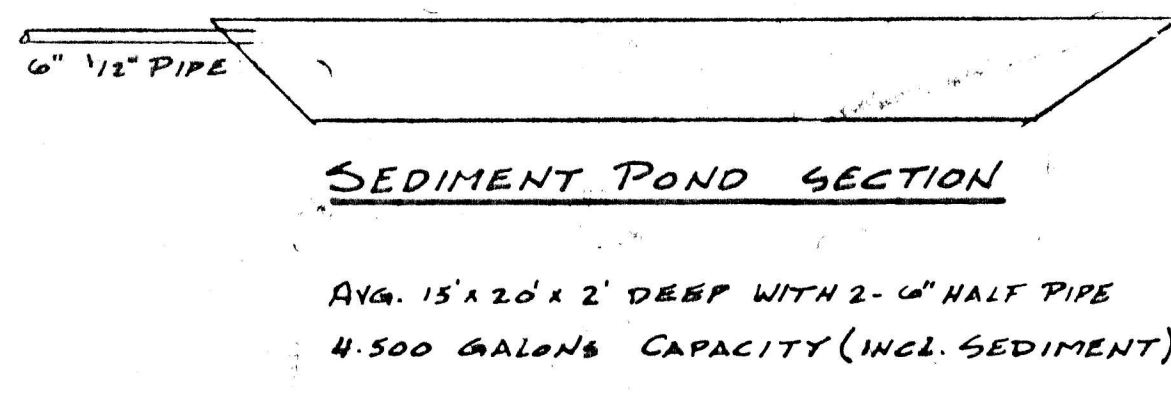
CISTERN / OF RUNOFF

1 UNITS 5-14 INCL. TO EASTERN BUILDING SWALE → DISIPATOR BOX 1
 2 UNITS 15-23 & 25-42 TO CENTRAL SWALE → DISIPATOR BOX 2
 DISIPATOR BOX 1 & 2 TO SEDIMENT POND - O/F & CROSS S. BUILDING

SIZING
 ASSUMPTIONS: a) 100 YEAR RAIN - 5. Flg 1 Hr INTENSITY = 4.75", ST. X 6" / HR.
 b) CONCRETE LINED SWALES
 c) 1/2 UNITS IN OVERFLOW CONDITION
 d) RATIONAL FORMULA - (LOGICAL APPROACH)

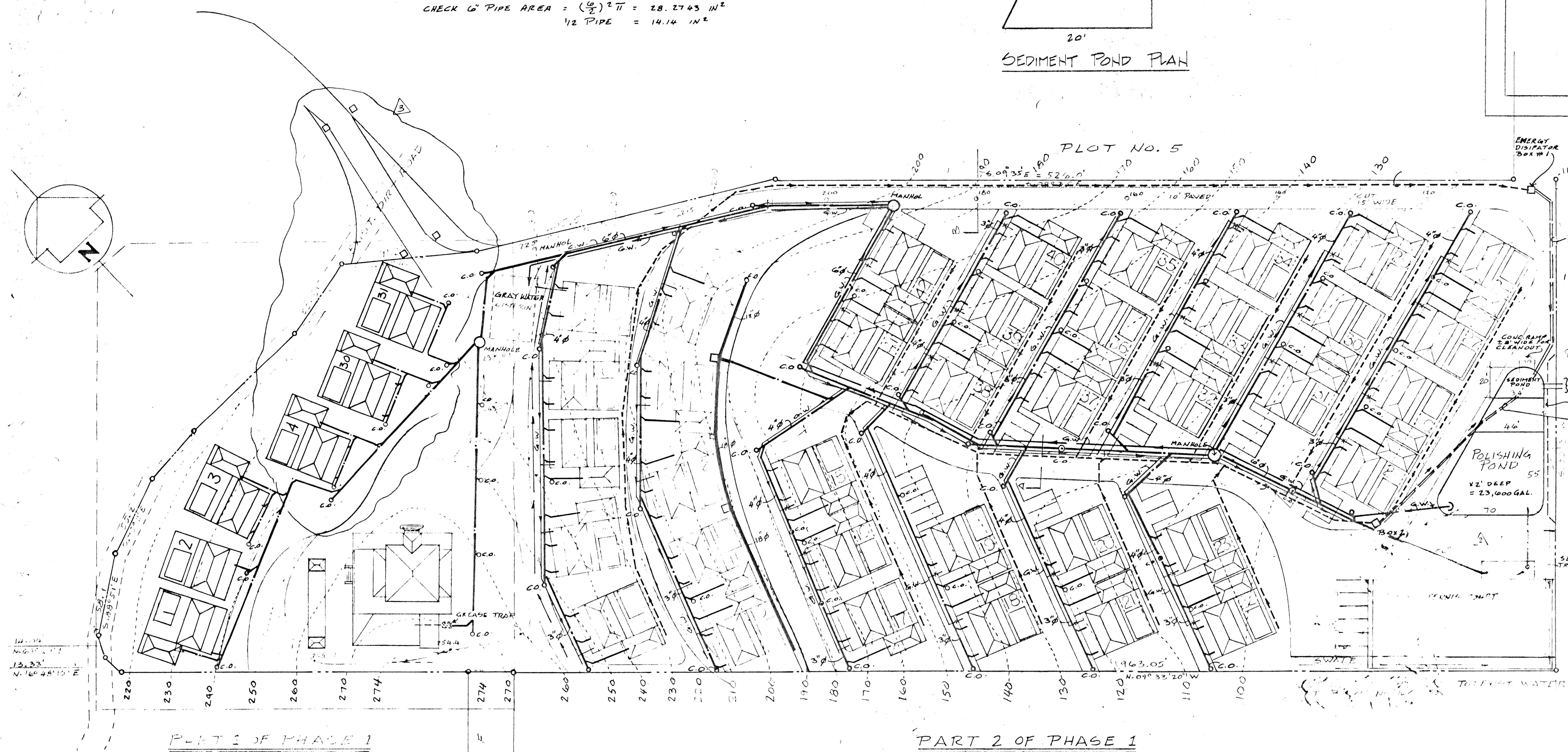
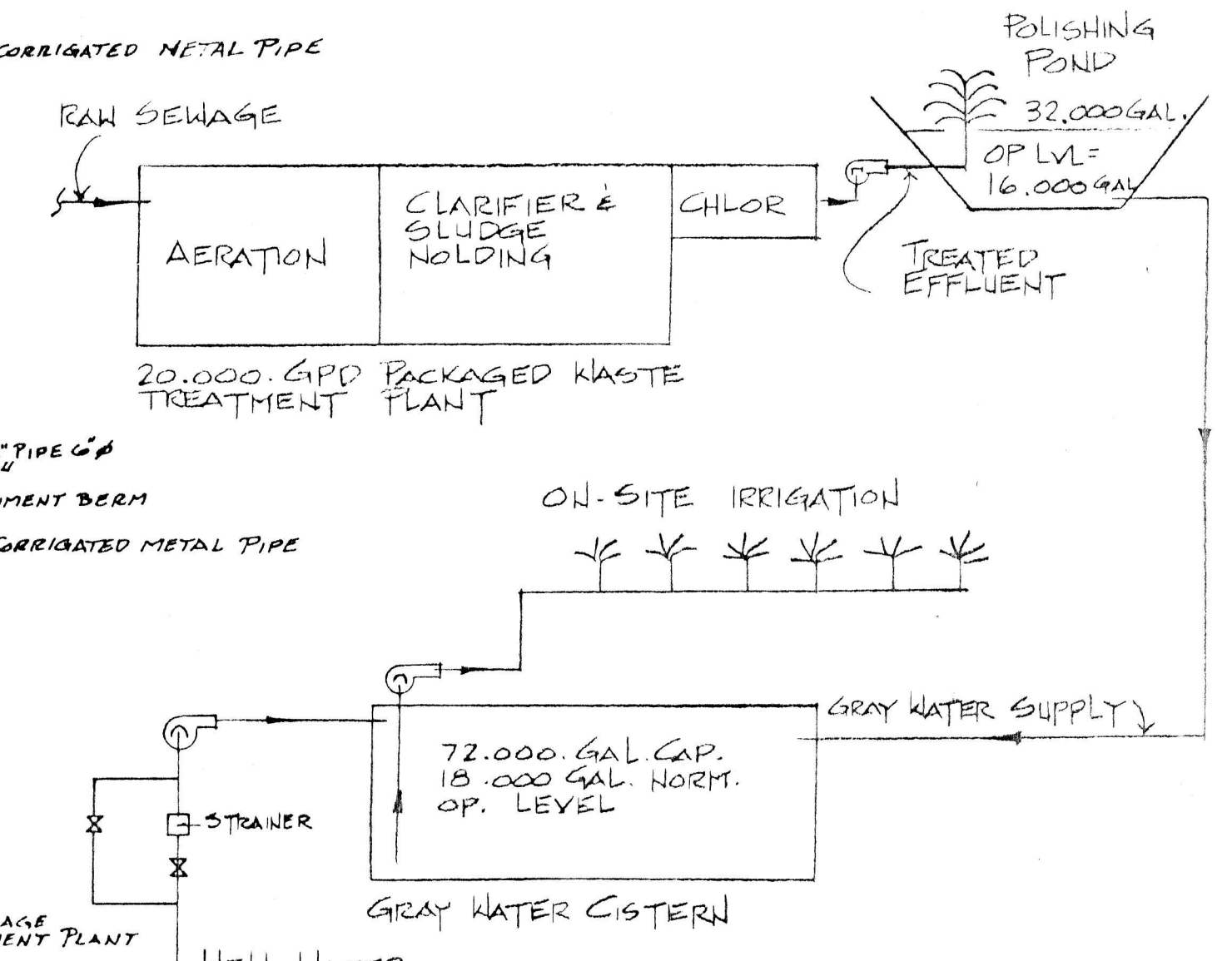
$Q = A \cdot I$
 $Q = \text{RUNOFF: PEAK DISCHARGE OF WATERSHED IN CUBIC FEET / SEC. CFS. DUE TO MINIMUM STORM ASSUMED}$
 $A = \text{WATERSHED IN ACRES}$
 $I = \text{INTENSITY OF RAINFALL IN INCHES / HOUR BASED ON CONCENTRATION TIME}$
 1 HOUR INTENSITY EXPECTED TO BE 6" ONLY IN 100 YEARS.
 LENGTH OF CENTRAL STRIP (WORST CONDITION) - 12 UNITS DISCHARGE
 RUNNING ± 70' 10" AT - 11% SLOPE.

1 UNIT AREA (ROOF) = 25' x 35' + 23' x 12' = 1214 SQ. FT. x 1/2 = 14568
 AREA OF INFLUENCE (WORST CASE) 43,540 ± = 0.33 AC. L = 70
 CHARACTER OF CONDUIT = PAVED
 SLOPE = 11% ± CONCENTRATION TIME = 2 MINS ± I = 6"
 $A = 0.33 \text{ AC.}$
 $C = .90 \text{ TO } 1.00 - \text{SAT } 0.95$
 $I = 6"$
 $Q = .33 \times .95 \times 6 = 1.88 \text{ C.F.S.}$
 $V = \text{VELOCITY (DESIGN MAXIMUM 25 F.P.S.)}$
 $Q = X \text{ SECTION AREA OF CHANNEL}$
 $Q = Va \text{ \& } a = \frac{Q}{V} = \frac{1.88}{25} = .075 \text{ SQ. FT.} = \frac{10.83}{144} \text{ IN}^2$
 ASSUMING 1/2 PIPE Ø RUNNING FULL AREA $O = \pi r^2$
 $\text{PIPE RADIUS } r = \sqrt{\frac{10.83 \times 144}{\pi}} = 2.6258"$
 $\text{DIAMETER} = 2.6258 \times 2 = 5.25"$
 ∴ 1/2 - 6" PIPE WILL SUFFICE.
 CHECK 6" PIPE AREA = $(\frac{6}{2})^2 \pi = 28.2743 \text{ IN}^2$
 1/2 PIPE = 14.14 IN^2



WASTE WATER SCHEMATIC FLOW DIAGRAM

N.T.S.



CARIBBEAN CONSTRUCTION SERVICES AND ASSOCIATES

DATE: 6/17/08

SCALE: 1" = 4'-0"

PROJECT: M.A.L. 13, 20, 00

VILLA MADELEINE

JOB NO. 1232, 1235

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